Time: 3 Hours Total Marks: 80

N.B: (1) Question No. 1 is compulsory.

- (2) Attempt any three questions out of remaining five questions.
- (3) Assume suitable data wherever required and state it clearly.
- (4) Illustrate your answers with neat component sketches wherever required.
- (5) IS 1343:1980, IS 3370 and relevant codes require for design are permitted.
- 1. Design a post tensioned pre-stressed concrete slab bridge deck for a national 32 highway crossing to suit the following data:

Clear span = 10 m
Live load = IRC class AA traced vehicle width of bearing = 400 mm
Clear width of roadway = 7.5m
footpath 1m on either side kerbs = 500 mm wide thickness of wearing coat = 80mm type of structure = class 1 type

Materials M40 grade concrete and 6 mm diameter high tensile wires with an ultimate tensile strength of  $1200 \text{ N/mm}^2$  housed in cables with 12 wires and anchored by freyssinet anchorages of 150 mm diameter. For supplementary reinforcement, adopt Fe 415 grade HYSD bars. Compressive stress at transfer, fci =35 N/mm², loss ratio =0.8. The permissible compressive stresses in concrete at transfer and working loads are  $f_{ct} = 15 \text{ N/mm}^2$ ,  $f_{cw} = 12 \text{ N/mm}^2$ ,  $f_{tt} = f_{tw} = 0$ 

## OR

Design a post tensioned roof girder as a class I structure using following data Effective span = 45 m

Live load = 9 kN/m

Dead load (excluding self-weight) = 2 kN/m

Load Factors (For dead load = 1.4, for live load 1.6) Cube strength of concrete,

 $fcu = 50 \text{ N/mm}^2$ 

Cube strength at transfer fci = 35 N/mm<sup>2</sup>

Tensile strength of concrete ft =  $1.7 \text{ N/mm}^2$ 

Modulus of elasticity of concrete Es =  $34 \text{ kN/mm}^2$ , Loss ratio = 0.85

8 mm diameter high tensile wires having a characteristic tensile strength fpu = 1200 N/mm<sup>2</sup> are available for use.

The modulus of elasticity of high tensile wires is 200 kN/mm<sup>2</sup>.

**2.** a) What is concordant cable profile? Explain its advantages

provided with a pre-stressed concrete ring beam.

A reinforced concrete dome of 30 m base diameter and a rise of 3. 5 m is to be designed for a pre-stressed concrete cylindrical tank. The shell dome to be

Design the dome and thering beam for a superimposed load of  $1.5 \text{ kN/m}^2$ . The 5 mm diameter high tensile wires initially stressed to  $1000 \text{ N/mm}^2$  are available for pre-stressing the ring beam. The loss ratio is 0.80 and permissible compressive stress in concrete at transfer is  $14 \text{ N/mm}^2$ . Assume radius of the shell dome = 32 m, thickness of shell = 75 mm, semi central angle  $\alpha = 28^{\circ} 4'$ .

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- 3. a) A prestressed beam with rectangular cross section with a width of 150 mm and depth of 300 mm is continuous over two spans AB = BC = 10m. The cable with zero eccentricity at the ends and an eccentricity of 50mm towards the top fibers of the beam over the central support, carries an effective force of 550kN.
  - (i) Calculate the secondary moments developed at B.
  - (ii) If the beam supports the concentrated load of 25 kN each at mid points of the span, evaluate the resultant stresses at the central support section B.

Also locate the position of pressure line at the section.

b) Explain stages of prestressing

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**4.** a) Write a short note on design of prestressed concrete liquid storage tank.

6

- b) A composite beam of rectangular section is made up of a pre-tensioned inverted T-beam having a slab thickness and width of 150 and 1050mm respectively The rib size is 150 mm by 800 mm The cast in situ has thickness and width of 1000mm with the modulus of elasticity of 30 kN/mm<sup>2</sup> If the differential shrinkage is 100×10<sup>-6</sup> units, estimate the shrinkage stresses developed in the precast and cast in situ units.
- 5. a) Explain the concept of load balancing.

4

- b) Design a non-cylinder prestressed concrete pipe of 500 mm internal diameter to withstand a working hydrostatic pressure of 1.05 N/mm², using a 2.5 mm high tensile wire stressed 1000 N/mm² at transfer. Permissible maximum and minimum stresses in concrete at transfer and service loads are 14 and 0.7 N/mm². The loss ratio is 0.8. Calculate also the test pressure required to produce a tensile stress of 0.7 N/mm² in concrete when applied immediately after tensioning and also the winding stress in steel if ES = 35 kN/mm²
- **6.** a) Explain methods of prestressing of shell structure

4

b) Design a post-tensioned pre-stressed concrete two-way slab,  $6m \times 9$  m with discontinuous edges, to support an imposed load of  $3 \text{ kN/m}^2$ . Cables of four wires of 5 mm diameter carrying an effective force of 130 kN are available for use. Design the spacing of cables in the two directions and check for the safety of the slab against collapse and excessive deflection at service loads. Assume (B. M coefficients for short span = 0.089, = 0.056 for long span),  $fck = 40 \text{ N/mm}^2$ ,  $fp = 1500 \text{ N/mm}^2$  and  $Ec = 38 \text{ kN/mm}^2$ 

2'----<u>-</u>\$'-----

1 ime:	3 H	lours		46)	70,	Total Marks: 8	5 <b>U</b>	
N.B.	1 2 3 4	Question No. 1 is Compulsory. Solve any three questions from reads Assume suitable data wherever reduced of relevant IS codes is permitted.	quired	~ V		arly.	3.7.7.697	
		A9**	77.		1580	187. 187.		
Q. 1	Att	ttempt any <b>FOUR</b> of the following				83, 24		
	a) b)	Explain Airy's theory for design of Explain the difference between the w.r.t. various points.		rs and sile	os in the	systematic manne	r :	
	c)	± 4	stics fea	itures of y	ields lines	40	25/	
	d)						to, :	
	e)			71.6	Tob			
Q.2	do:	esign a silo for storing wheat with dimome has Central opening 50cm. Use A eel. For Wheat take density = 7850 Nowing reinforcement details.	Airy's T	heory. Us	se M20 c	oncrete and Fe415	5	
					70,			
Q.3	a)	Derive the expression relating yield for Simply supported equilateral tria					. 1	
	b)		= (0)	~ Y	27.69	200 J	;	
Q.4	dis of	Design a strip footing for row of 4 column of size 350mm square. Center to center distance between the adjacent columns is 5.5m. The two exterior columns carry a load of 1800 kN each and two interior columns carry a load of 2200 kN each. Assume SBC of soil as 280 kN/m <sup>2</sup> . Use M20/Fe415.						
			49)					
Q.5	R.O hal ava Us fra	esign an intermediate portal frame for C.C. slab supported on a rectangular pull is 4.0 m from the floor level. The vailable at 1.7 m below the ground leve se Limit State Method. Draw neat sketcame. The live load may be assumed to kN/m <sup>2</sup> . Use concrete of M-25 grade an	ortal fra plinth cl. The siches sho be 4.5 k	mes at 5 m height is tructure is wing deta N/m <sup>2</sup> and	n c/c. The 0.50 m and single stood its of reins	clear height of the nd the firm soil is preyed. forcement in porta	e s	
		ST ST ST	5					
Q.6		esign either square or circular bunker a inforcement for the bunker to store 350 Unit weight of wheat: 7850 /m³; Angle of repose: 30°	0 kN of	wheat usin	ng the foll	_	e 2	
	Lie	The stored wheat is to be surcharged at its angle of repose. Use M-20 concrete and HYSD steel of grade Fe 415.						
	De	esign either square or circular bunker. inforcement.			ketch show	wing details of the	3	
# FF	304	48 P	e 1 of	e 1				
. v	304		Je i Ai					

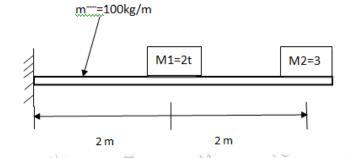
$\mathbf{D}$	urat	ion: 3hrs [Max Marks: 80]	290
		A A A	2
In	stru	ections:	3
		(1) Question No.1 is Compulsory.	7
		(2) Attempt any <b>three</b> questions out of the remaining five.	
		(3) Each full question carries 20 marks.	É
		(4) Assume suitable data, if required and state it clearly.	
			250
Q.1		Briefly explain the following: (any FOUR)	(5)
<b>V.1</b>	a	Compatibility conditions	05M
	b	Octahedral stresses	05M
	c	Energy methods for the solution of buckling problems	05M
	d		05M
		Biharmonic equation in Cartesian coordinates	> \.
	e	Torsional buckling	05M
<b>Q.2</b>	a	The state of plane stress at a point is given by:	12M
		$\sigma_x = 35 \text{Mpa.}$ $\sigma_y = 50 \text{MPa}$ and $\tau_{xy} = -35 \text{MPa}$ .	4
		Determine the principal stresses, the maximum shearing stress and the octahedral	15
	, (	shearing stress.	3
	b	The displacement components of a body are	08M
	ar'	$u_x = 2x + y$ ; $u_y = z$ ; $u_z = z - y$	,\
4	5'	determine strain in the direction of $1/\sqrt{3}$ , $1/\sqrt{3}$ , $1/\sqrt{3}$	
,0		46 45 46 45 46 45	
Q.3	a	Derive the equation of deflection of a cantilever beam having length L carrying a	12M
		concentrated load P at the free end.	
,	3h	The components of the strain tensor at a point in a body are given by $\varepsilon_x = 0.001$ ,	08M
A	7		00111
(9)		$\varepsilon_y = 0.003$ , $\varepsilon_z = \gamma_{xy} = 0$ , $\gamma_{yz} = 0.001$ , $\gamma_{xz} = -0.004$ . If the modulus of elasticity	
40		$E = 2 \times 10^5 \text{ N/mm}^2$ and the Poisson's ratio is 0.28, determine the components of the	
O'	(5	stress tensor. Also determine Lame's constant.	
) ~	4		
Q.4	a	Derive Euler's expression for buckling load of a column with both ends hinged.	12M
20	√b	Derive the equation for torsion of rectangular bars of thin cross section.	08M
1,00			
Q.5	a	A prismatic column of length L and flexural rigidity EI carries an axial compressive	15M
9 <sup>v</sup>	1	force P and a central lateral load W. Derive the expression for the midspan deflection	
	4	and maximum bending moment. One end of the beam column is hinged and the other	
, (	2	end is supported on rollers.	
A	Ъ	State Bettis's Reciprocal theorem. What are its applications?	05M
20			
Q.6		Write short notes on:	0.53.5
, i	a	Plane stress and plane strain problems	05M
4	ь	Prandtl's membrane analogy	05M
5	c	Lateral buckling of beams	05M
(8)	d	Hydrostatic and deviator stress tensors	05M
ET .			
	(		

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(3 Hours) Total Marks: 80

## N.B:

- 1. Attempt any FOUR questions.
- 2. Assume suitable data wherever necessary.
- 3. Figures to the right indicate full marks.
- 1(a) Explain the terms, magnitude and intensity of earthquake. How it is measured?
- (b) What is damping? Explain the various the various types of damping. What are the effects of 5 damping on the structure?
- (c) State the different methods of seismic analysis as per IS 1893-2002. Also state under what conditions these methods are used.
- (d) What is transmissibility of a system? Briefly explain how vibration isolation can be achieved.
- 2 (a) Derive the expression for steady state response of damped SDOF system subjected to harmonic force defined by  $P_{(t)} = P_0 \cos \omega t$ .
- (b) For the beam shown in figure, calculate the fundamental frequency using Rayleigh's method.  $10 E=2x10^5 Mpa$ ,  $I=9x10^7 mm^4$



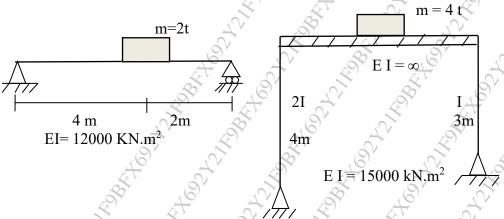
A three storey single bay frame has same storey height. The storey stiffness are  $K_1 = K_2 = K_3 = 15$  K, and the lumped mass on each storey is  $m_1 = 2m$ ,  $m_2 = 1.5m$  and  $m_3 = m/2$ . Calculate natural frequencies and mode shapes. Also find the normal modes and verify the orthogonality principle.

5

(b) What are the ductility provisions in the RCC buildings as per IS 13920.

## Paper / Subject Code: 59022 / Structural Dynamics

4(a) For the structural systems shown in the figure, compute the natural frequency of vibration.



- (b) A machine of 1000 kg mass supported on springs of total stiffness 1000 kN/m and has an unbalanced rotating element which results in a disturbing force of 500 N at a speed of 1000 rpm. Assuming 5% and 10% damping, calculate
  - i, Its amplitude of motion.
  - ii. Transmissibility and
  - iii. Force transmitted to foundation.
- (c) What is an earthquake? Explain how the earthquakes are classified based on their causes.
- 5(a) Starting from first principle, derive the expression for frequency of vibration of a simply supported beam of span L, flexural rigidity EI and uniform mass 'm' kg/m.
- (b) Explain the concept involved in carrying out modal analysis of structure subjected to dynamic loads in MDOF system.
- 6 (a) A three storey frame with the free vibration characteristics are given below is subjected to a suddenly applied constant load of 30 KN at 2<sup>nd</sup> floor level & 20KN at 3<sup>rd</sup> floor level, determine maximum displacement of each storey.

Storey Storey		Mass	Mass	Mode shapes		
Level	Height	No.	(t)	T = 0.29	T = 0.12	T = 0.10
8	(m)	₹V		sec	sec 💎	sec
0	7	50	70,	Mode 1	Mode 2	Mode 3
Y (S	5		· / (	<b>(Φ1)</b>	(Φ2)	(Ф3)
First	3	1	25	0.404	-0.676	2.959
Secon	3	2	25	0.716	-0.526	-2.578
de la	A	2	2501	40		
Roof	3	30	22	1.0	1.0	1.0

State and prove orthogonality principle.

\*\*\*\*\*\*\*

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(b)